SOLUM™
CONSULTANTS LTD.

GEOTECHNICAL & MATERIAL
TESTING LABORATORY

November 1, 2005

Environmental Engineering Company
Suite 1, 2345-6 Street, N.W.
Calgary, AB
T7T 8Z8

Attention: John Smith

EEC Project #: 12345678

Geotechnical Laboratory Testing Report

SOLUM Job #: 100010501024
Received: October 24, 2005
# of Samples Received: One-5 gal pail, 1 Shelby Tube

Test	Quantity	ASTM Designation
Water Content	1	D2216
Atterberg Limits	1	D4318
Full Gradation	1	D422
Standard Proctor	1	D698
Fall Cone Shear Test	1	N/A
Vane Shear Test	1	D4648
Hydraulic Conductivity; Remolded at 95% of Maximum Dry Density	1	D5084

Saad A.M. Farag
Laboratory Manager
Solum Consultants Ltd.

Total Cover Pages: 1

#9, 3620 29 Street NE, Calgary, AB T1Y 5Z8
Ph: (403) 250-3035 Fax: (403) 250-3021 Email: sfarag@solumconsultantsltd.com

Laboratory Analysis Summary

SOLUM CONSULTANTS LTD.
GEOTECHNICAL & MATERIAL TESTING LABORATORY

#9, 3620 - 29 Street, NE
Calgary, Alberta T1Y 5Z8
Ph: (403)250-3035
Fax: (403)250-3021
Email: solum@mts.net
www.solumconsultantsltd.com

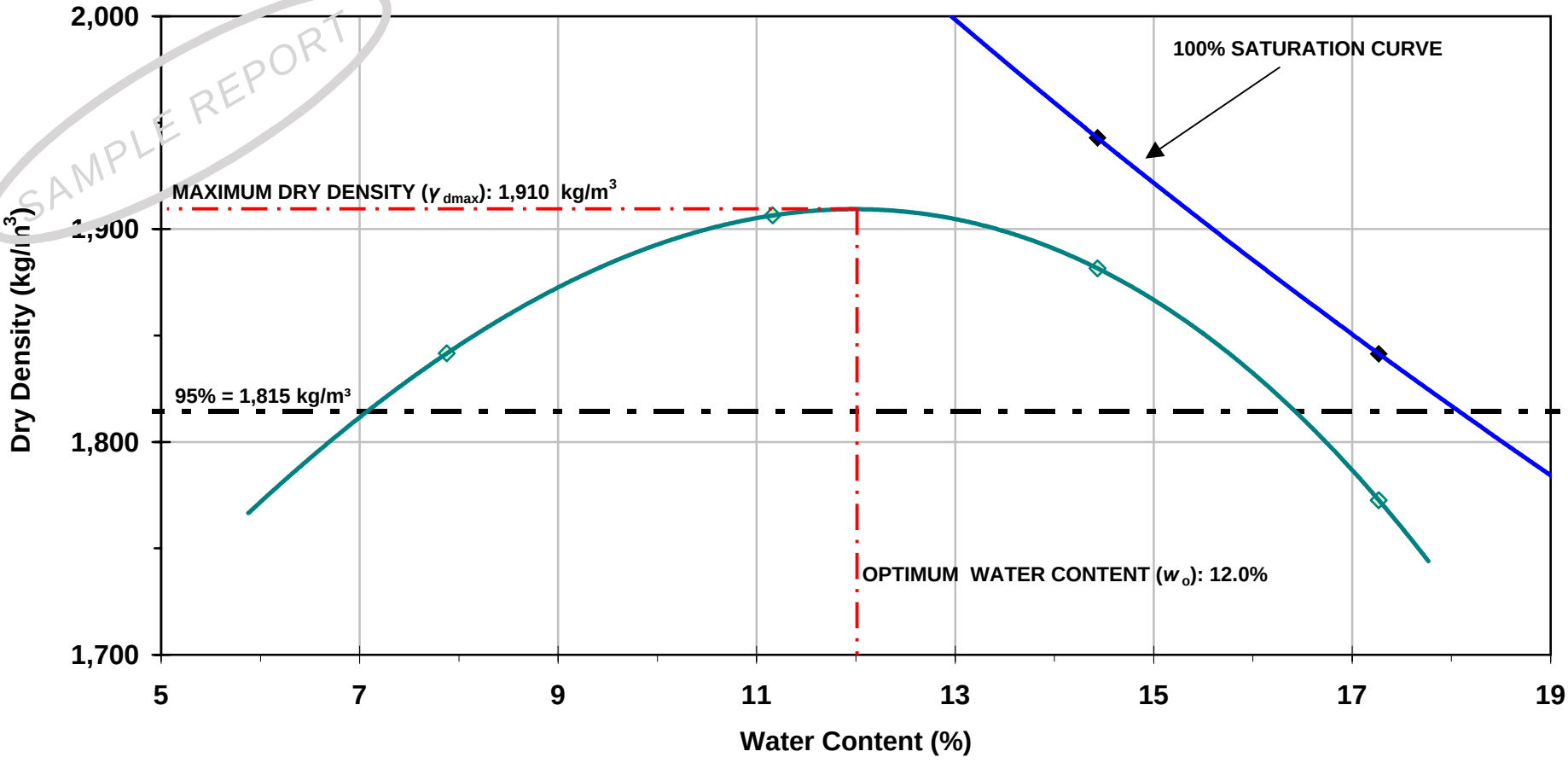
Project Number:	12345678
Client:	Geotechnical Engineering Company
Project Name:	Site X Geotechnical Investigation
Location:	---
Tested By:	KC/SF
Reviewed By:	
Date Reviewed:	25-Oct-09 (dd-mm-yy)

Results

Sample ID	Depth (m)	Moisture Content(%)	Atterberg Limits				Particle Size Analysis					Soil Classification** Group Symbols	Standard Proctor				Constant Head Permeability k_{20} (cm/sec)
			Liquid Limit(%)	Plastic Limit(%)	Plastic Index(%)	Classification* (USCS)	Cobble Size (%) (75-300mm)	Gravel Size (%) (4.75-75mm)	Sand Size (%) (0.075-4.5mm)	Silt Size (%) (0.005-0.075 mm)	Clay Size (%) (<0.005mm)		Maximum Dry Density (kg/m^3)	Optimum Water Content (%)	Over Size Density Correction (kg/m^3)	Over Size Optimum Water Content Correction(%)	
CCL-68680200-05	GR	40.7	83	45	38	MH	0.0	0.8	32.6	35.9	30.7	MH	1068	46.0	N/A	N/A	3.6E-08

* Note: Soil classification is for material less than 0.425 mm (material used for Atterberg Limits), this includes the fine sand, silt and clay fraction of the sample. The USCS does not recognize the group symbol CI.

** Note: Soil classification is for the whole sample. Soil classification uses the Atterberg Limits results and the percent fines, percent sand and percent gravel as described in ASTM D2487.



Laboratory Compaction Characteristics of Soil - Standard Proctor (ASTM D698)

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Site X Remediation

Client: Environmental Engineering Company
Project No.: 12345678
Sample ID: TP01
Depth: 0.5 - 1.5 m

OVERSIZE CORRECTION ASTM D4718
Particles Retained on the No. 4 (4.75 mm) Sieve (P_c): **2.3 %**
G_M: **2.55** (Assumed)
Corrected Max. Dry Density: **N/A**
Corrected Opt. Water Content: **N/A**

Tested by: **RA**
Date Tested: **26-Oct-05**
Approved by:

Atterberg Limits (ASTM D4318) - Method A

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GEOTECHNICAL & MATERIAL TESTING LABORATORY

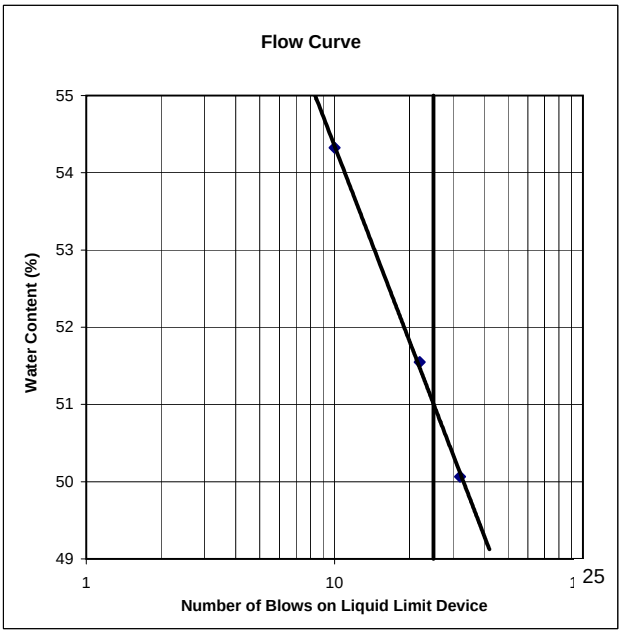
Project Number:	12345678		
Client:	Geotechnical Engineering Company		
Project Name:	Site X Geotechnical Investigation		
Location:	Calgary, AB		
Sample ID	TP01	Depth:	2.37-2.67 m
Tested By:	KC	Reviewed By:	SF
Date Tested:	29-Nov-11	(dd-mmm-yy)	

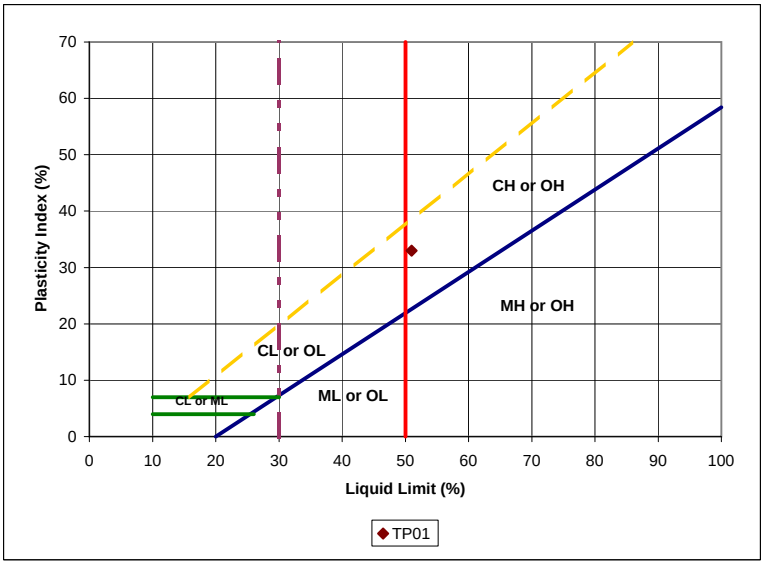
Sample Information

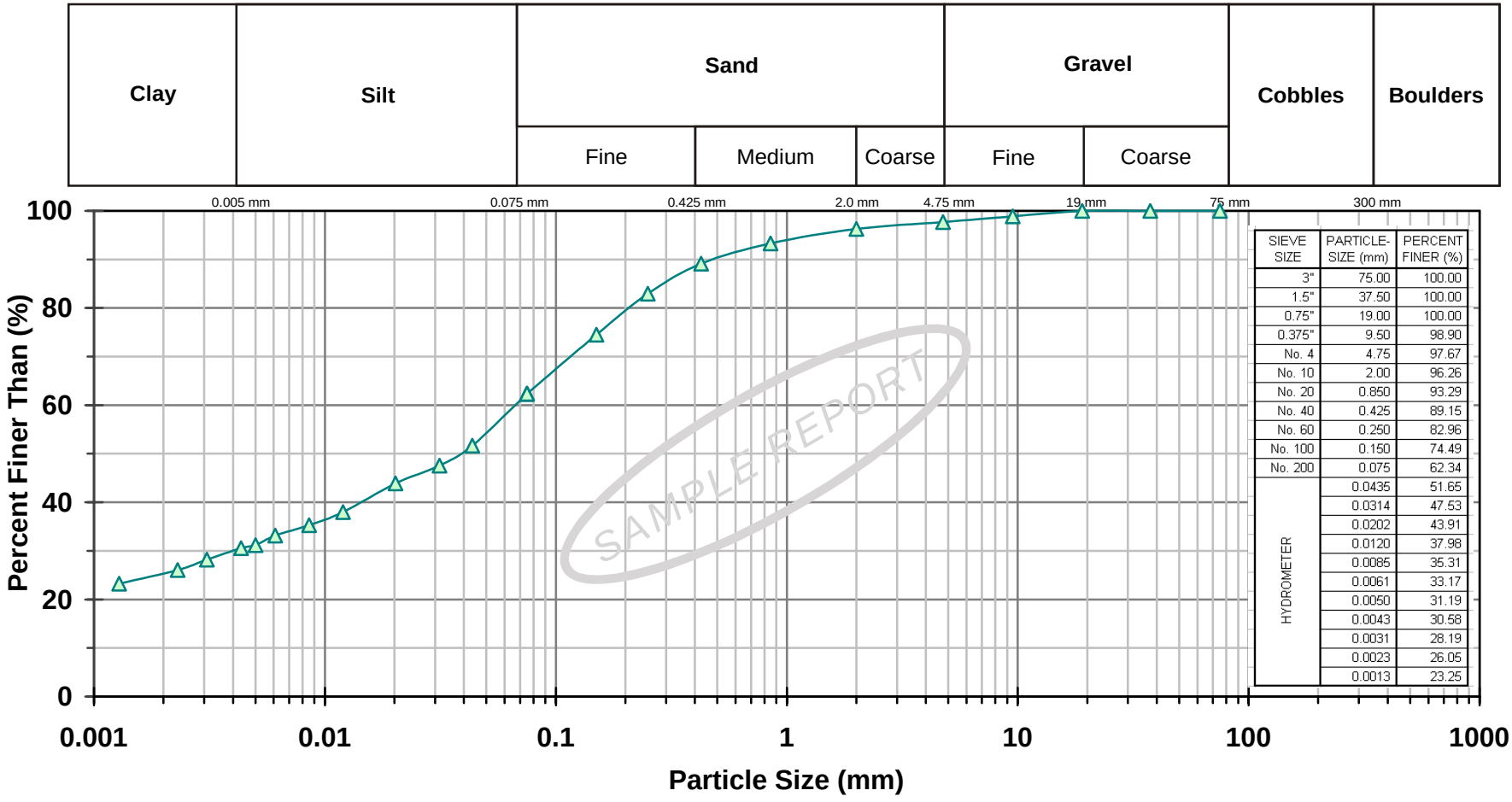
	Liquid Limit (Air Dried) - Multipoint Method			Liquid Limit (Oven Dried)	
Container ID	1	2	3		
Number of Blows	10	22	32		
Wet Sample Weight +Tare (g)	20.70	22.21	22.78		
Dry Sample Weight +Tare (g)	17.37	18.55	18.97		
Weight of Water (g)	3.33	3.66	3.81		
Tare (g)	11.24	11.45	11.36		
Weight of Dry Soil (g)	6.13	7.10	7.61		
Water Content (%)	54.3	51.5	50.1		

	Plastic Limit	
Container ID	4	5
Wet Sample Weight +Tare (g)	30.43	26.46
Dry Sample Weight +Tare (g)	27.90	24.55
Weight of Water (g)	2.53	1.91
Tare (g)	14.05	13.95
Weight of Dry Soil (g)	13.85	10.60
Water Content (%)	18.3%	18.0%
Average Water Content (%)	18.1	

Results	
Liquid Limit (Air Dried) (%)	51
Liquid Limit (Oven Dried) (%)	---
LL % Difference	---
Plastic Limit (%)	18
Plasticity Index (%)	33
-40 Mesh Sieve (y/n)	y
Unified Soil Classification System	CH







Particle Size Analysis (ASTM C136/D422)

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Site X Remediation

Client: Environmental Engineering Company

Project No.: 12345678.00

Sample ID: TP01

Depth: 0.5 - 1.5 m

Particle Size (%)

Cobbles:	0.0
Gravel:	2.3
Sand:	35.3
Silt:	31.2
Clay:	31.2

Tested by: **KC**

Date Tested: **25-Oct-05**

Approved by: **RA**

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Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter (ASTM D5084)

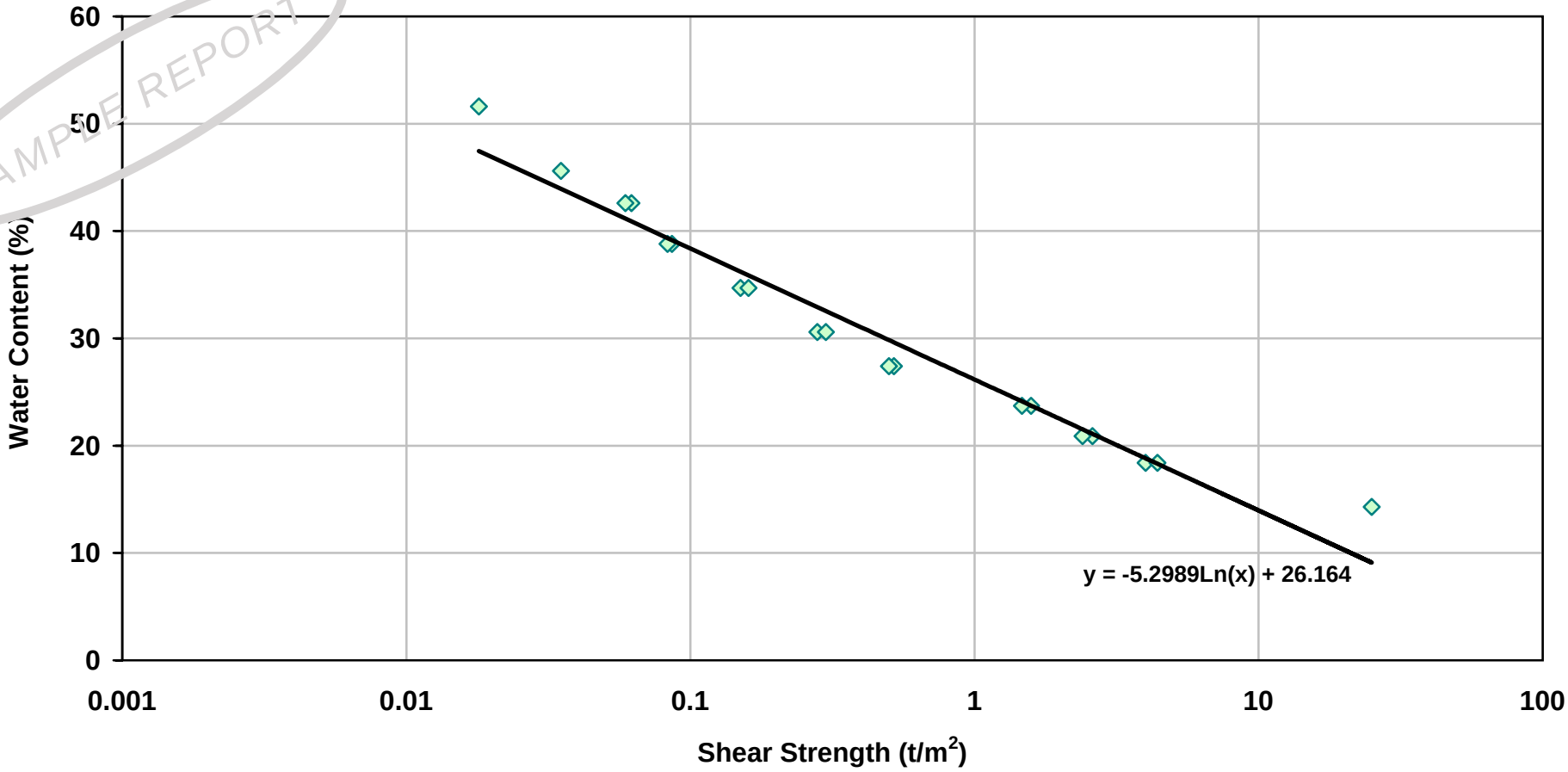
Client:	*Envoironmental Engineering Company*	**Borehole No.:**	**Tested by: KC / SF**
Project No.:	*12345678*	**Sample No.:**	*TP01*
Project Name:	*Site X Remediation*	**Depth of Sample:**	*4.0-4.5 m*
Location:	*Calgary, AB*	**Date of Testing:**	*17-Jan-08* *(dd -mm -yy)*

DATE	TIME	ELAPSED TIME	INCREMENTAL TIME		TEMP.	Rt	IN BURRETS				OUT BURRETS				Hydraulic Conductivity K_{20} (cm/sec)
		(MIN)	(MIN)	(SEC)	(deg. C)		READING	VOLUME (cm^3)	CHANGE (cm^3)	TOTAL (cm^3)	READING	VOLUME (cm^3)	CHANGE (cm^3)	TOTAL (cm^3)	
27-Jan-08	0:01				17	1.079	20	41.28			20	41.28			
	2:30	149.00	149	8940	17	1.079	19.7	40.66	0.62	0.62	21.2	43.76	2.48	2.48	3.95E-08
	5:00	299.00	150	9000	17	1.079	19.4	40.04	0.62	1.24	21.4	44.17	0.41	2.89	3.94E-08

Reported **3.9E-08** **cm/sec**

Wet Mass:	257.37 gr	**Top Pore Pressure:**	385 kPa	**Sample Height :**	3.33 cm
Water Content:	27.3%	**Bottom Pore Pressure:**	400 kPa	**Sample Diameter :**	7.24 cm
Wet Density:	1880.7 kg/m^3	**Cell Pressure:**	415 kPa	**Sample Area :**	41.13 cm^2
Dry Density:	1477.3 kg/m^3	**Hydraulic Gradient:**	46.0	**Sample Volume :**	136.85 cm^3

Remarks:



Shear Strength vs. Water Content (Fall Cone Test)

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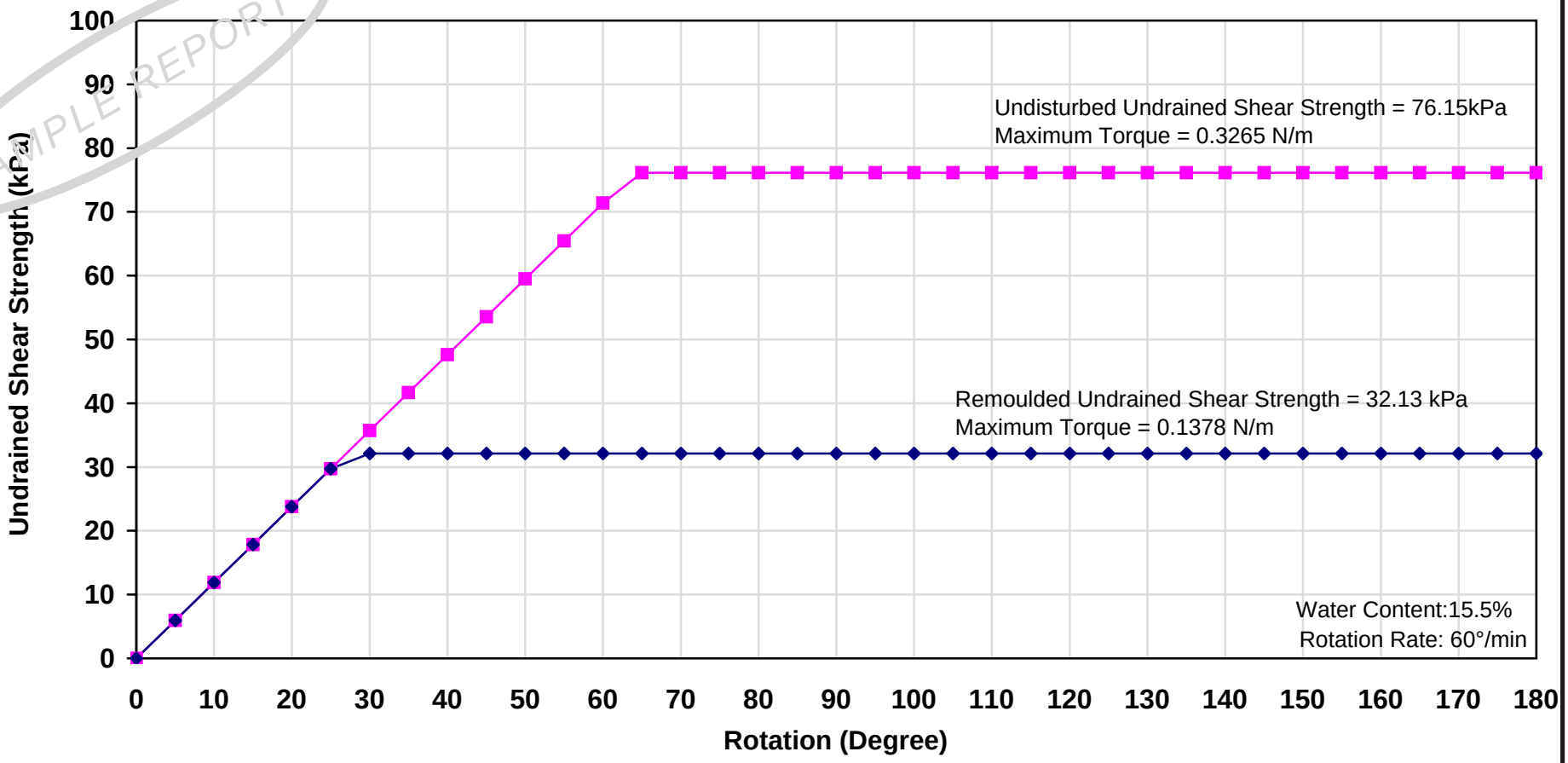
Site X Remediation

Client:	Environmental Engineering Company
Project No.:	12345678
Sample ID:	TP01
Depth:	0.5 - 1.5 m

Tested by: RA

Date Tested: 26-Oct-05

Approved by:



Laboratory Miniature Vane Shear Test for Saturated Fine-Grained Clayey Soil - ASTM D4648

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GEOTECHNICAL & MATERIAL TESTING LABORATORY

Site X Remediation

Client:	Environmental Engineering Company
Project No.:	12345678
Sample ID:	TP01
Depth:	0.5-1.5 m

Tested by: RA

Date Tested: 26-Oct-05

Approved by:

Boring No.	N/A	Job No.	N/A
Sample No.	B1	Job Name	N/A
Depth (ft)	5.0 m	Tested By	SF/KC
Date	11/5/2010	Calculated By	KC
Sheet No.		Checked By	SF

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UNCONFINED COMPRESSION TEST (ASTM D2166)

SPECIMEN DIAMETER	7.34 (cm)	MOISTURE CONTENT	21.0 (%)
SPECIMEN HEIGHT	15.48 (cm)	MASS OF WET SAMPLE	1273.1 (g)
SPECIMEN AREA	42.27 (cm^2)	HEIGHT/DIAMETER RATIO	2.11
SPECIMEN VOLUME	654.41 (cm^3)	WET BULK DENSITY	1945.4 (kg/m^3)
ASSUMED Gs	2.75	INITIAL SATURATION	81.3 (%)
INITIAL VOID RATIO	0.71		

Shear Strength 76.36 (kPa)
Axial Failure Strain 9.47 (%)

Unconfined Compressive Strength 152.72 (kPa)

VERT. DISPL. (cm)	LOAD CELL (kN)	ε1 (%)	σ1-σ3 (kPa)	(σ1-σ3)/2 (kPa)
0.00	0.00	0.00	0.00	0.00
0.13	0.16	0.85	37.25	18.62
0.28	0.29	1.84	67.46	33.73
0.41	0.39	2.64	88.83	44.41
0.57	0.49	3.71	111.16	55.58
0.74	0.56	4.81	126.12	63.06
0.91	0.62	5.87	137.59	68.79
1.08	0.66	6.99	145.45	72.73
1.30	0.70	8.37	151.01	75.50
1.47	0.71	9.47	152.72	76.36
1.60	0.72	10.30	152.16	76.08
1.75	0.68	11.29	142.71	71.36
1.84	0.59	11.86	123.18	61.59

Boring No.	N/A	Job No.	N/A
Sample No.	B1	Job Name	N/A
Depth (ft)	5.0 m	Tested By	SF/KC
Date	11/5/2010	Calculated By	KC
Sheet No.		Checked By	SF

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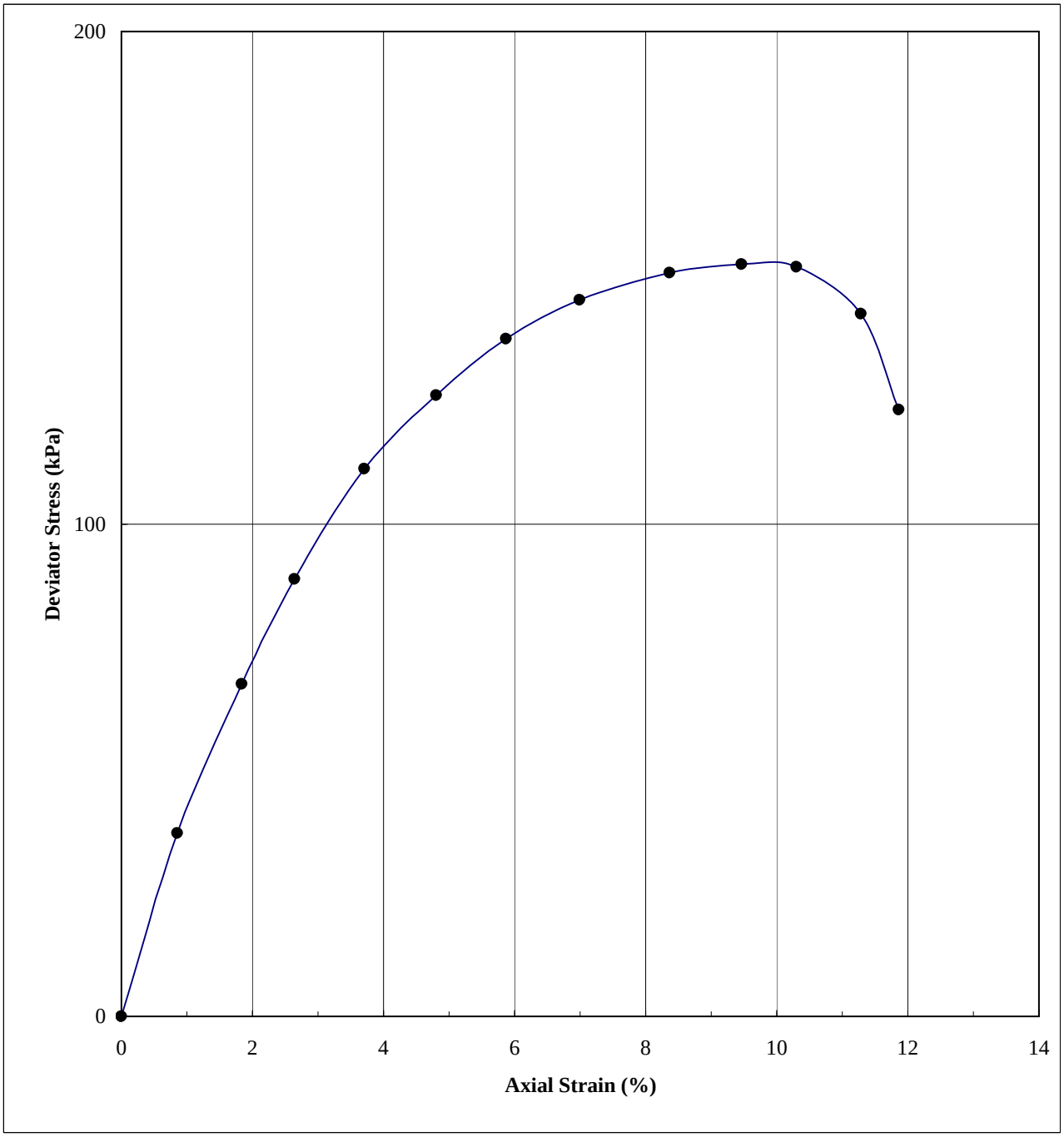
UNCONFINED COMPRESSION TEST (ASTM D2166)
Stress-Strain Plot



Direct Shear Test of Soils Under Consolidated Drained Conditions (ASM D3080)

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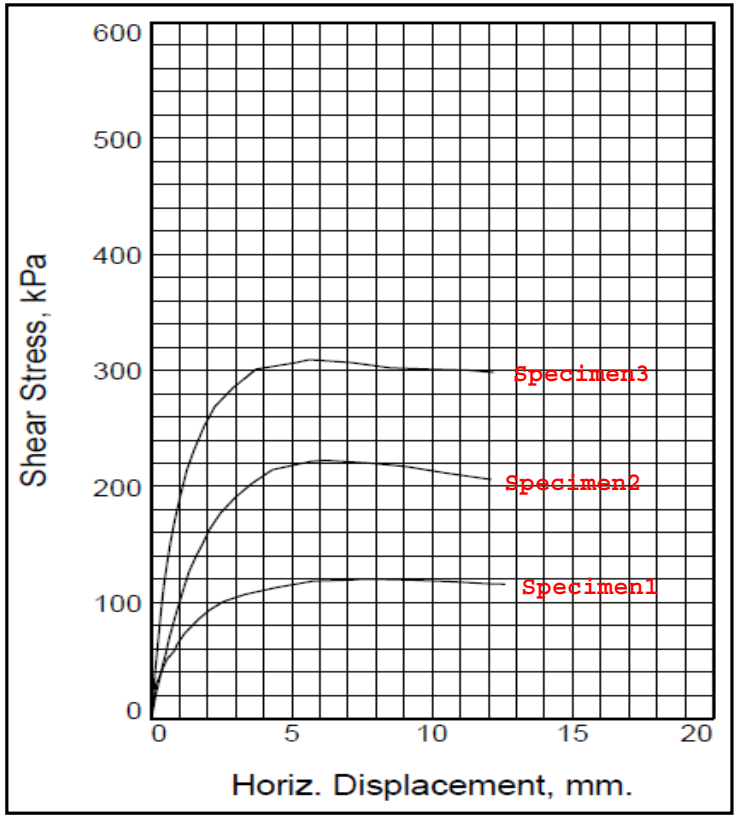
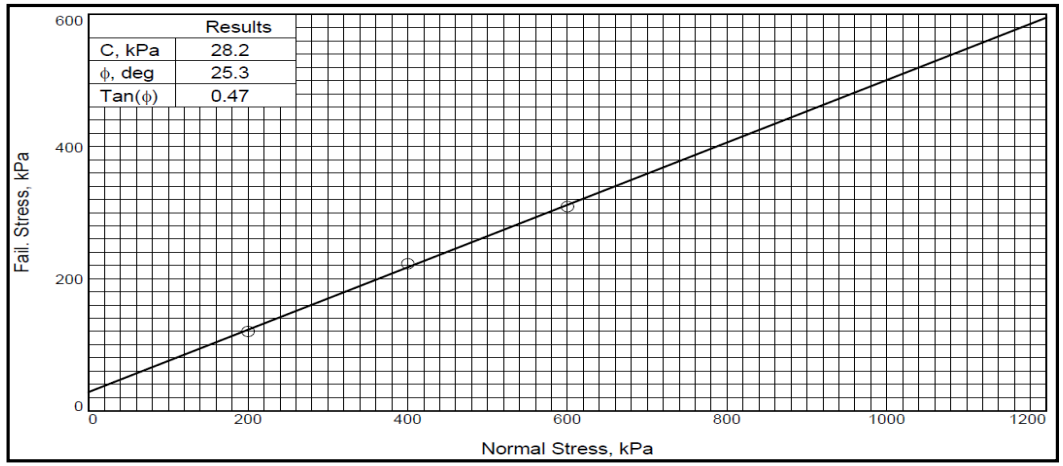
Project Number:	12345678		
Client:	Geotechnical Engineering Company		
Project Name:	Site X Geotechnical Investigation		
Location:	---		
Sample ID:	BH-01	Depth:	N/A
Tested By:	SF/KC	Reviewed By:	SF
Date of Testing:	20-Nov-09	(dd-mmm-yy)	

Sample Information

Sample Preparation Method: [X] Remoulded

Specimen No.	Normal Stress (kPa)	Peak Stress(kPa)	Residual Stress(kPa)	Horizontal Disp. (mm)	Shearing Rate (mm/min)	Add Water
1	200	120.1	---	7.5	0.1270	Yes
2	400	222.5	---	6.2	0.1270	
3	600	309.1	---	5.6	0.1270	





SWELL/CONSOLIDATION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source:
Sample No.: B1
Elev. or Depth: **Sample Length(in./cm.):**
Location:
Description:
Liquid Limit: **Plasticity Index:**
USCS: **AASHTO:** **Figure No.:**
Testing Remarks:

Test Specimen Data

TOTAL SAMPLE	BEFORE TEST	AFTER TEST
Wet w+t = 57.83 g.	**Consolidometer # =** 1	**Wet w+t =** 169.47 g.
Dry w+t = 50.24 g.		**Dry w+t =** 145.83 g.
Tare Wt. = 14.05 g.	**Spec. Gravity =** 2.8	**Tare Wt. =** 43.00 g.
Height = 2.00 cm.	**Height =** 2.00 cm.	
Diameter = 6.18 cm.	**Diameter =** 6.18 cm.	
Weight = 124.43 g.	**Defl. Table =** reference Set (mm/kPa)	
Moisture = 21.0 %	**Ht. Solids =** 1.2247 cm.	**Moisture =** 23.0 %
Wet Den. = 2074 kg/m^3	**Dry Wt. =** 102.86 g.*	**Dry Wt. =** 102.83 g.
Dry Den. = 1715 kg/m^3	**Void Ratio =** 0.633	**Void Ratio =** 0.618
	Saturation = 92.8 %	

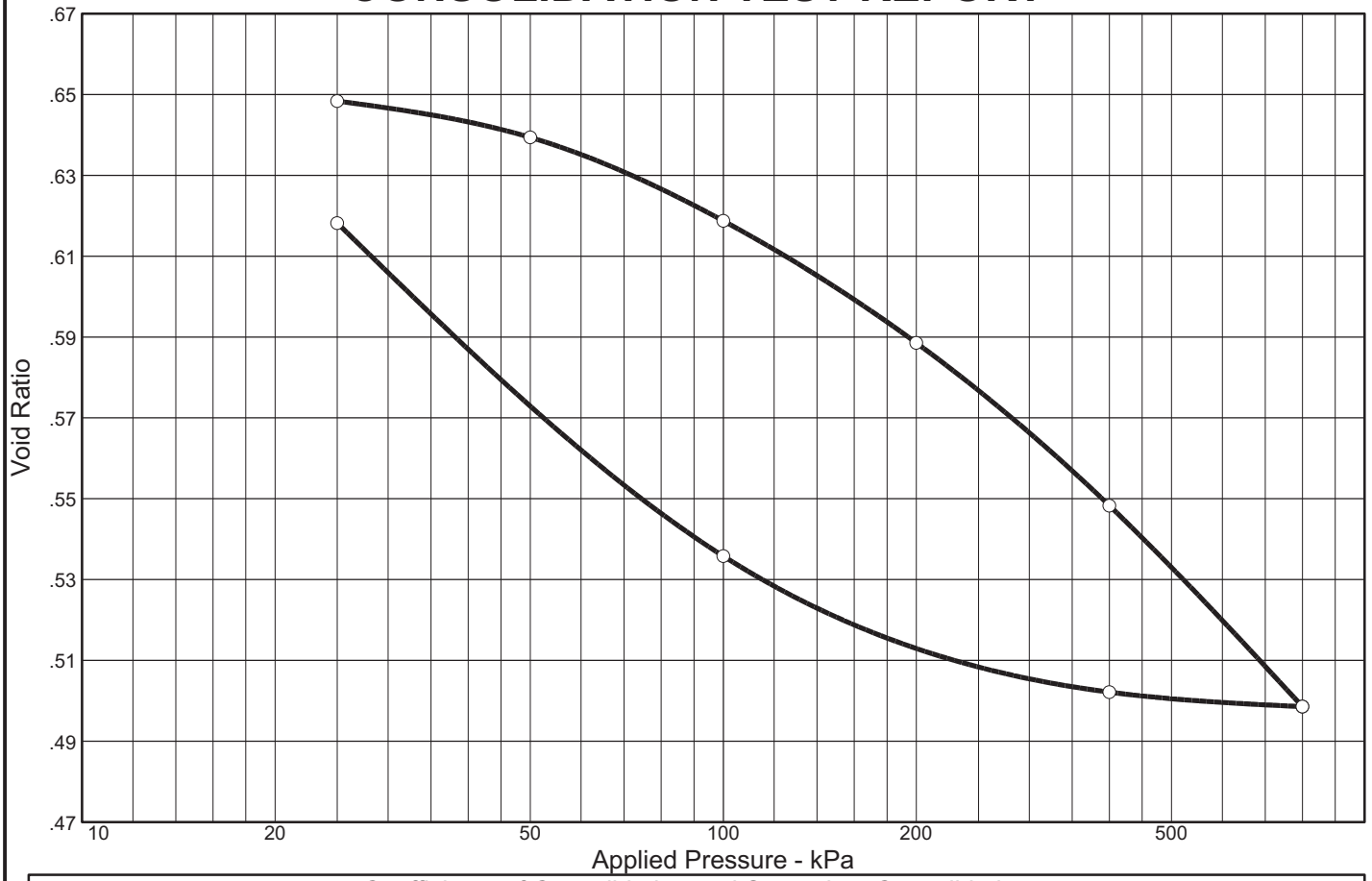
*** Initial dry weight used in calculations**

End-of-Load Summary

Pressure (kPa)	Final Dial (cm.)	Machine Defl. (cm.)	Corrected (cm.)	Void Ratio	% Compression /Swell
start	-0.01200			0.633	
25.00	0.00660	0.00010	0.00670	0.648	0.9 Swell
50.00	-0.00480	0.00050	-0.00430	0.639	0.4 Swell
100.00	-0.03040	0.00080	-0.02960	0.619	0.9 Comprs.
200.00	-0.06840	0.00180	-0.06660	0.589	2.7 Comprs.
400.00	-0.11860	0.00270	-0.11590	0.548	5.2 Comprs.
800.00	-0.18020	0.00340	-0.17680	0.499	8.2 Comprs.
400.00	-0.17540	0.00300	-0.17240	0.502	8.0 Comprs.
100.00	-0.13320	0.00200	-0.13120	0.536	6.0 Comprs.
25.00	-0.03040	0.00010	-0.03030	0.618	0.9 Comprs.

C_c = 0.17 **P_c =** 132.17 kPa **C_r =** 0.07

CONSOLIDATION TEST REPORT



Coefficients of Consolidation and Secondary Consolidation

No.	Load (kPa)	C_v (cm.^{2}/sec.)	C_α	No.	Load (kPa)	C_v (cm.^{2}/sec.)	C_α	No.	Load (kPa)	C_v (cm.^{2}/sec.)	C_α
2	50.00	0.0050									
3	100.00	0.0017									
4	200.00	0.0001									
5	400.00	0.0000									
6	800.00	0.0000									
7	400.00	0.0002									
8	100.00	0.0000									
9	25.00	0.0000									

Natural		Dry Dens. (kg/m^3)	LL	PI	Sp. Gr.	Overburden (kPa)	P_c (kPa)	C_c	C_r	Initial Void Ratio
Saturation	Moisture									
92.8 %	21.0 %	1715			2.8		132.17	0.17	0.07	0.633

MATERIAL DESCRIPTION	USCS	AASHTO

Project No. **Client:**

Project:

Source: **Sample No.:** B1 **Elev./Depth:**

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GEOTECHNICAL & MATERIAL TESTING LABORATORY

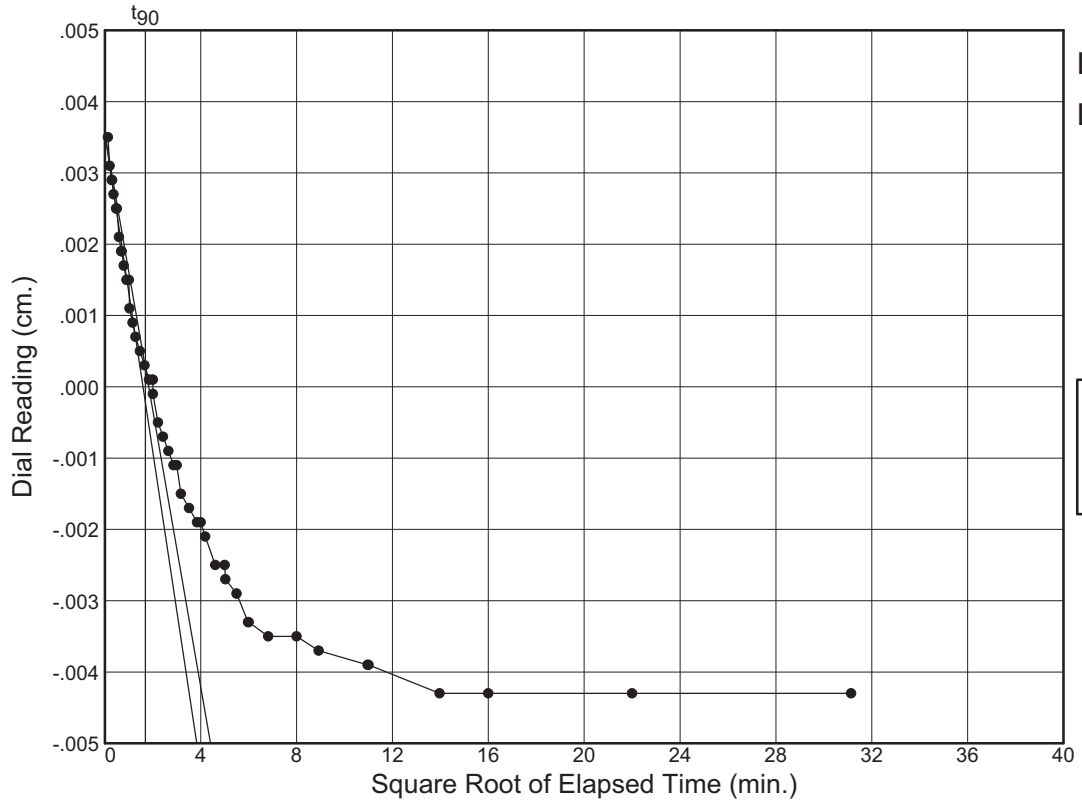
Remarks:

Figure

Dial Reading vs. Time

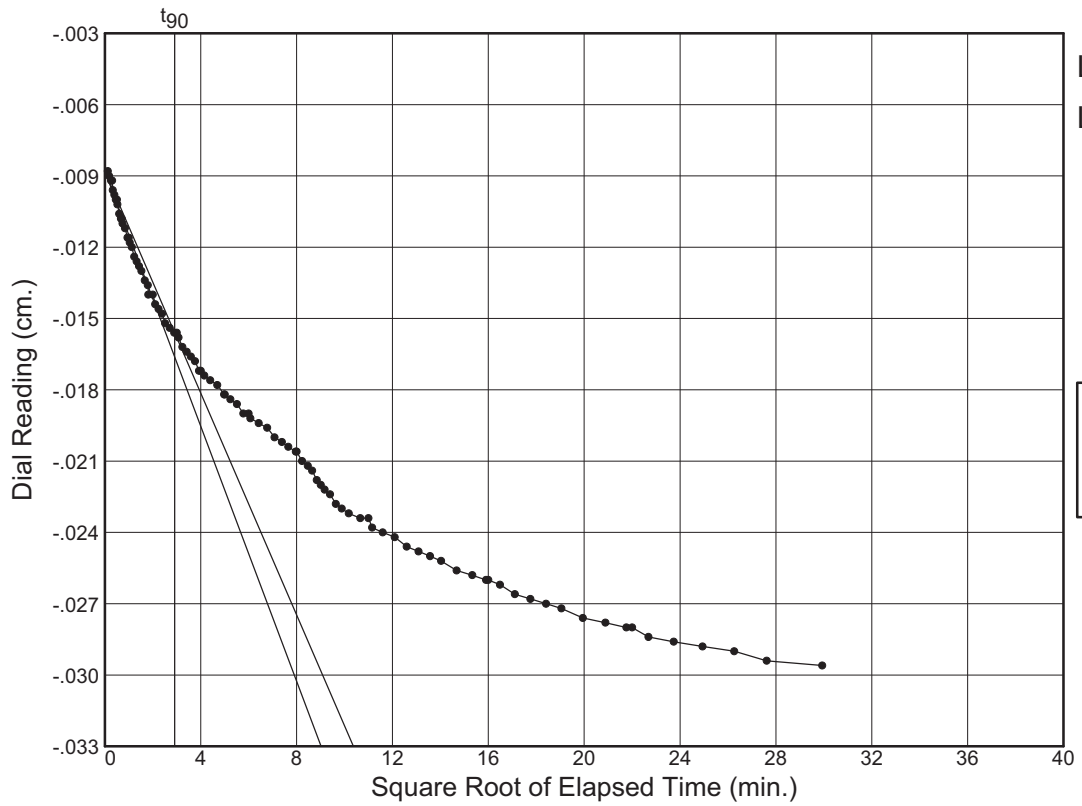
Project No.:
Project:

Source: Sample No.: B1 Elev./Depth:



Load No.= 2
Load= 50.00 kPa
D_0 = 0.00354
D_{90} = 0.00026
D_{100} = -0.00010
T_{90} = 2.86 min.

C_v @ T_{90}
0.0050 $cm.^2/sec.$



Load No.= 3
Load= 100.00 kPa
D_0 = -0.00881
D_{90} = -0.01560
D_{100} = -0.01635
T_{90} = 8.47 min.

C_v @ T_{90}
0.0017 $cm.^2/sec.$

Figure

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One-Dimensional Swell Test (ASTM D 4546)

JOB NAME N/A JOB NO. N/A

BORING NO. SAMPLE NO. BH10-01 DEPTH 1.50-2.50 m TEST DATE 12/6/2009

SAMPLE DESCRIPTION Silty clay w/ trace of gravels

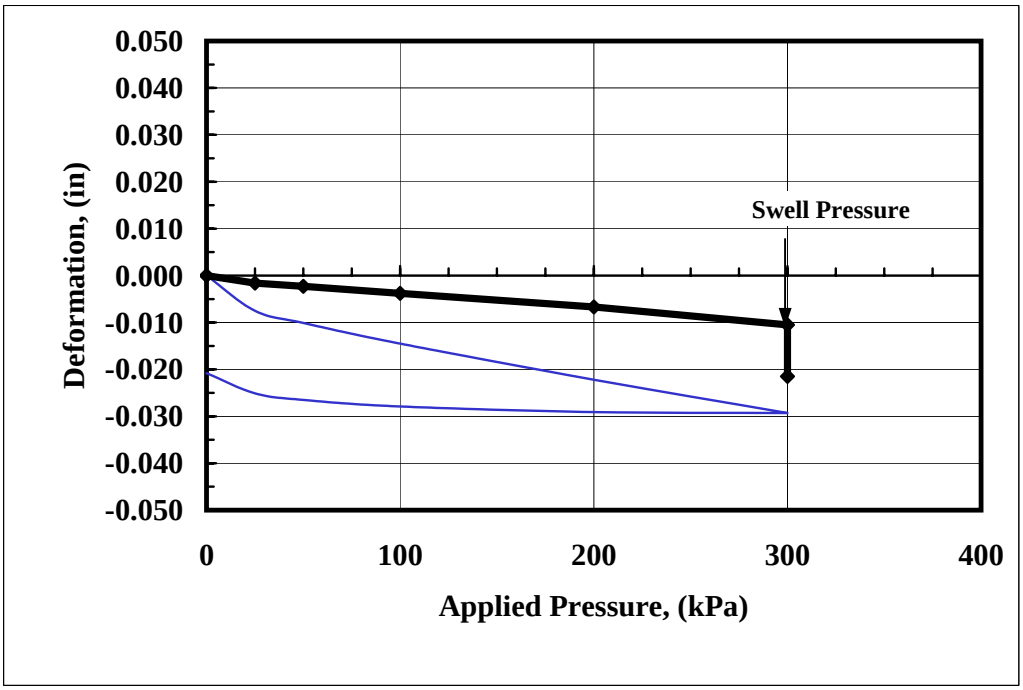
Loading (I)		Un-Loading		Loading (II)	
Applied Pressure (kPa)	Dial Reading (in)	Applied Pressure (kPa)	Dial Reading (in)	Applied Pressure (kPa)	Dial Reading (in)
0	0.0000	300	-0.0293	0	0.0000
25	-0.0075	200	-0.0291	25	-0.0016
50	-0.0101	100	-0.0279	50	-0.0023
100	-0.0145	50	-0.0265	100	-0.0038
200	-0.0222	25	-0.0251	200	-0.0067
300	-0.0293	0	-0.0208	300	-0.0105
				300	-0.0215

Dial Reading Before Inundate -0.0105 in

Dial Reading After Inundate -0.0215 in

Swell or Collapse Strain -1.1 (%)

Swell Pressure **300** **(kPa)**



Boring No.	N/A	Job No.	
Sample No.		Job Name	
Depth (m)		Tested By	SF & KC
Date		Calculated By	SF
Sheet No.		Checked By	SF

SOLUM CONSULTANTS LTD.

CONSOLIDATED-UNDRAINED TRIAXIAL COMPRESSION TEST
ASTM D4767

STAGE1

SPECIMEN DIAMETER	7.199 (cm)	CELL PRESSURE (CP)	565.0 (kPa)
SPECIMEN HEIGHT	13.376 (cm)	BACK PRESSURE (BP)	165.0 (kPa)
SPECIMEN AREA	40.678 (sq.cm)	EFFECTIVE PRESSURE (EP)	400.0 (kPa)
SPECIMEN VOLUME	544.092 (cu.cm)		

Failure Criteria: Max. Deviator Stress
Axial Failure Strain 1.52 (%)

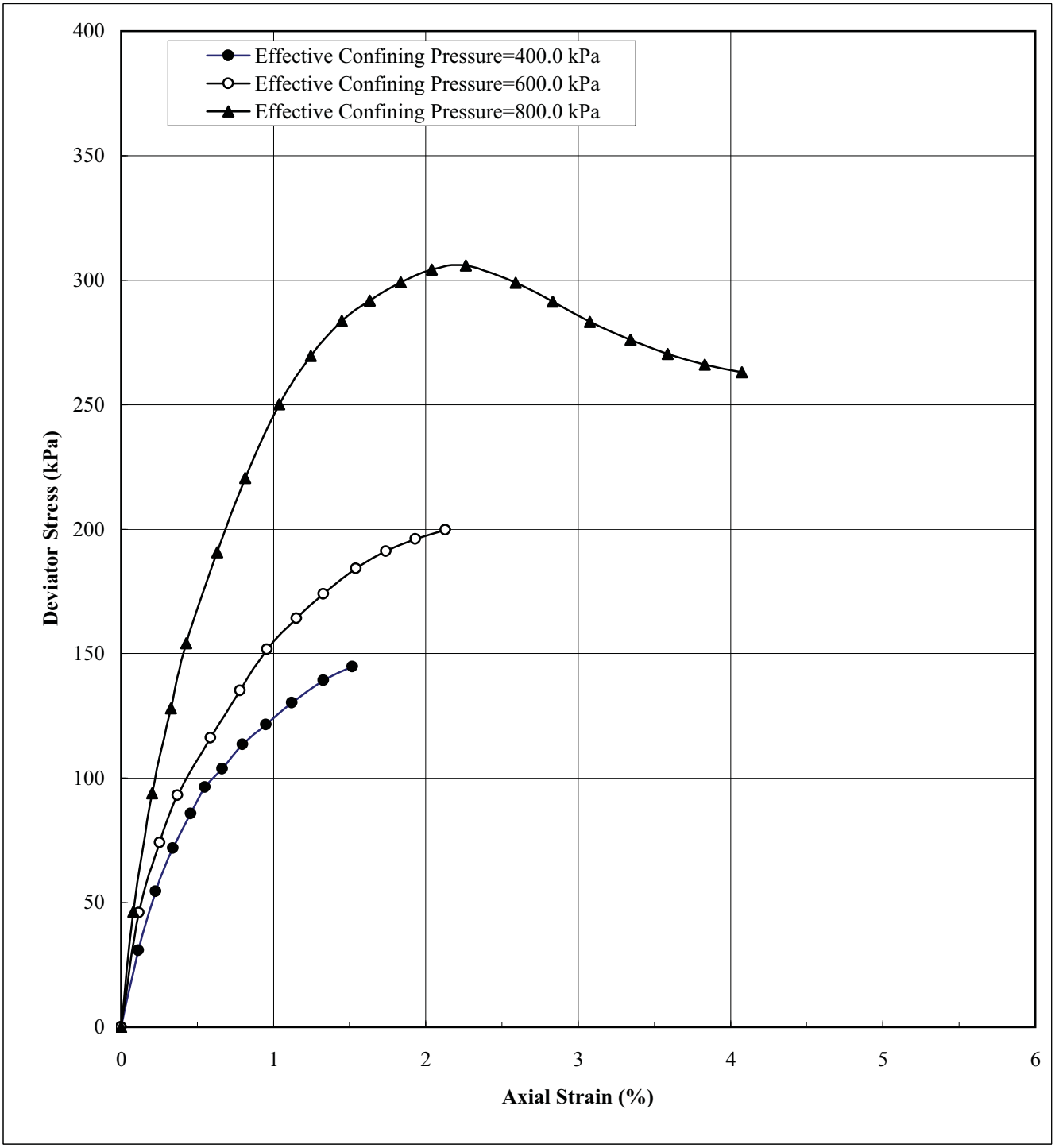
(σ1-σ3)f	144.74 (kPa)	**(σ3)f**	400.00 (kPa)	**(σ1)f**	544.74 (kPa)
(Δu)f	165.47 (kPa)	**(σ3')f**	234.53 (kPa)	**(σ1')f**	379.27 (kPa)

VERT. DISPL. (cm)	LOAD CELL (KN)	Δu (kPa)	ε1 (%)	σ1-σ3 (kPa)	σ1' (kPa)	σ3' (kPa)	p (kPa)	p' (kPa)	q (kPa)	σ1' /σ3'
0.00	0.00	0.00	0.00	0.00	400.00	400.00	400.00	400.00	0.00	1
0.02	0.13	15.86	0.11	30.80	414.94	384.14	415.40	399.54	15.40	1.08
0.03	0.22	35.85	0.23	54.55	418.70	364.15	427.28	391.42	27.28	1.15
0.05	0.29	55.16	0.34	71.93	416.77	344.84	435.96	380.80	35.96	1.21
0.06	0.35	72.39	0.46	85.67	413.27	327.61	442.83	370.44	42.83	1.26
0.07	0.39	86.87	0.55	96.35	409.48	313.13	448.18	361.30	48.18	1.31
0.09	0.42	102.04	0.66	103.63	401.59	297.96	451.81	349.77	51.81	1.35
0.11	0.47	117.21	0.80	113.58	396.37	282.79	456.79	339.58	56.79	1.40
0.13	0.50	128.24	0.95	121.42	393.18	271.76	460.71	332.47	60.71	1.45
0.15	0.54	144.10	1.12	130.29	386.19	255.90	465.15	321.05	65.15	1.51
0.18	0.57	155.82	1.33	139.19	383.37	244.18	469.59	313.77	69.59	1.57
0.20	0.60	165.47	1.52	144.74	379.26	234.53	472.37	306.89	72.37	1.62

Boring No.	N/A	Job No.	
Sample No.		Job Name	
Depth (m)		Tested By	SF & KC
Date		Calculated By	SF
Sheet No.		Checked By	SF

SOLUM CONSULTANTS LTD.

CONSOLIDATED-UNDRAINED TRIAXIAL COMPRESSION TEST
ASTM D4767



Boring No.	N/A	Job No.	
Sample No.		Job Name	
Depth (m)		Tested By	SF & KC
Date		Calculated By	SF
Sheet No.		Checked By	SF

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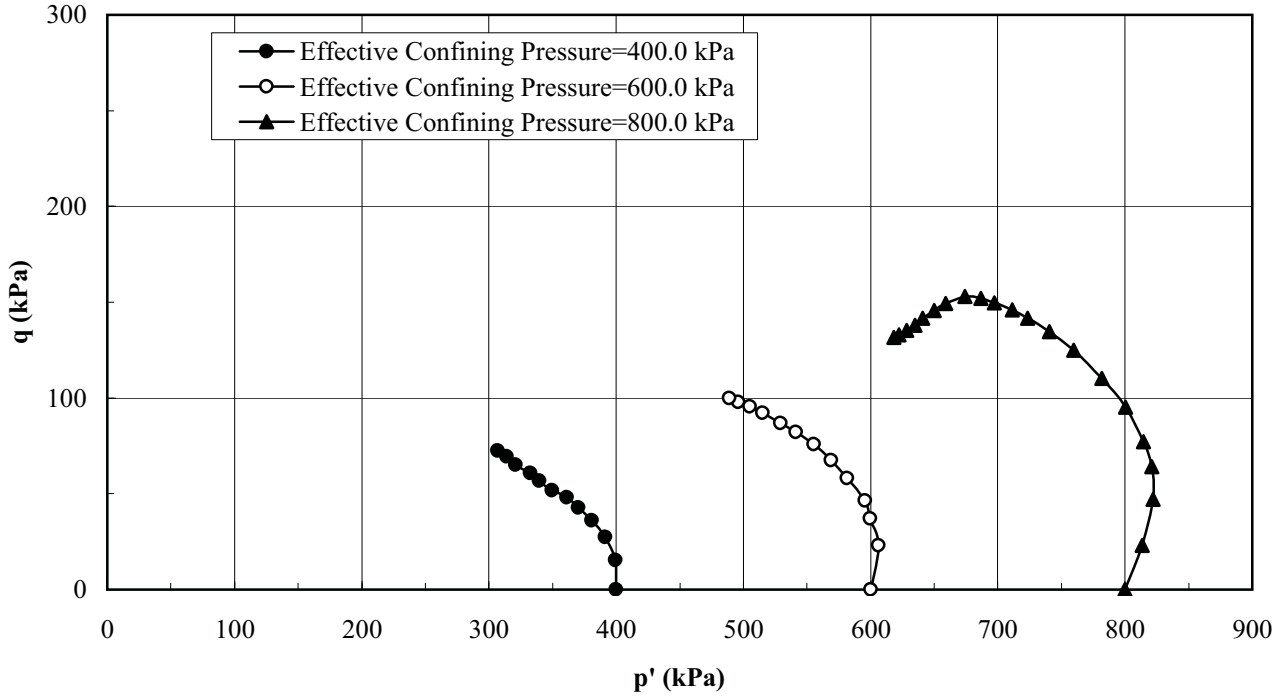
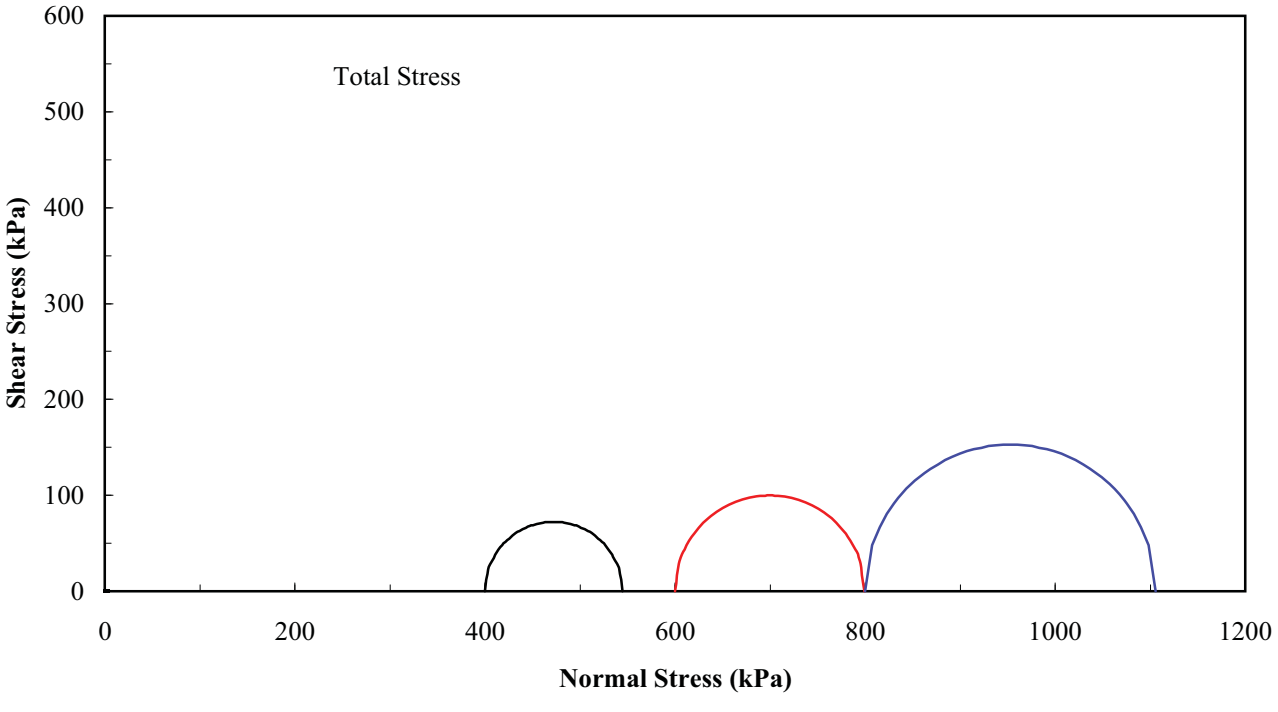
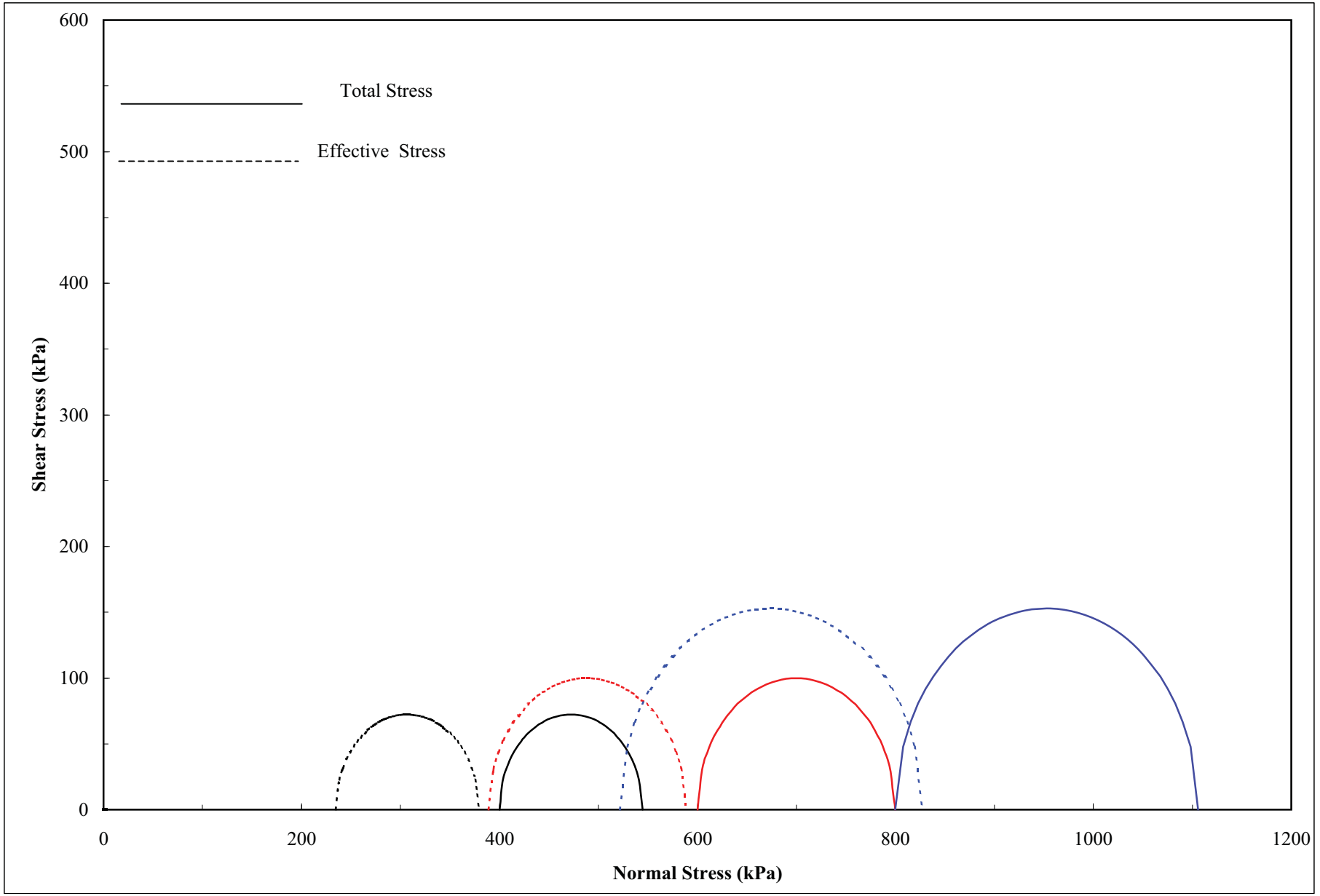
CONSOLIDATED-UNDRAINED TRIAXIAL COMPRESSION TEST
ASTM D4767






Total Stress
Effective Stress
Shear Stress (kPa)
Normal Stress (kPa)
0
100
200
300
400
500
600
0
200
400
600
800
1000
1200

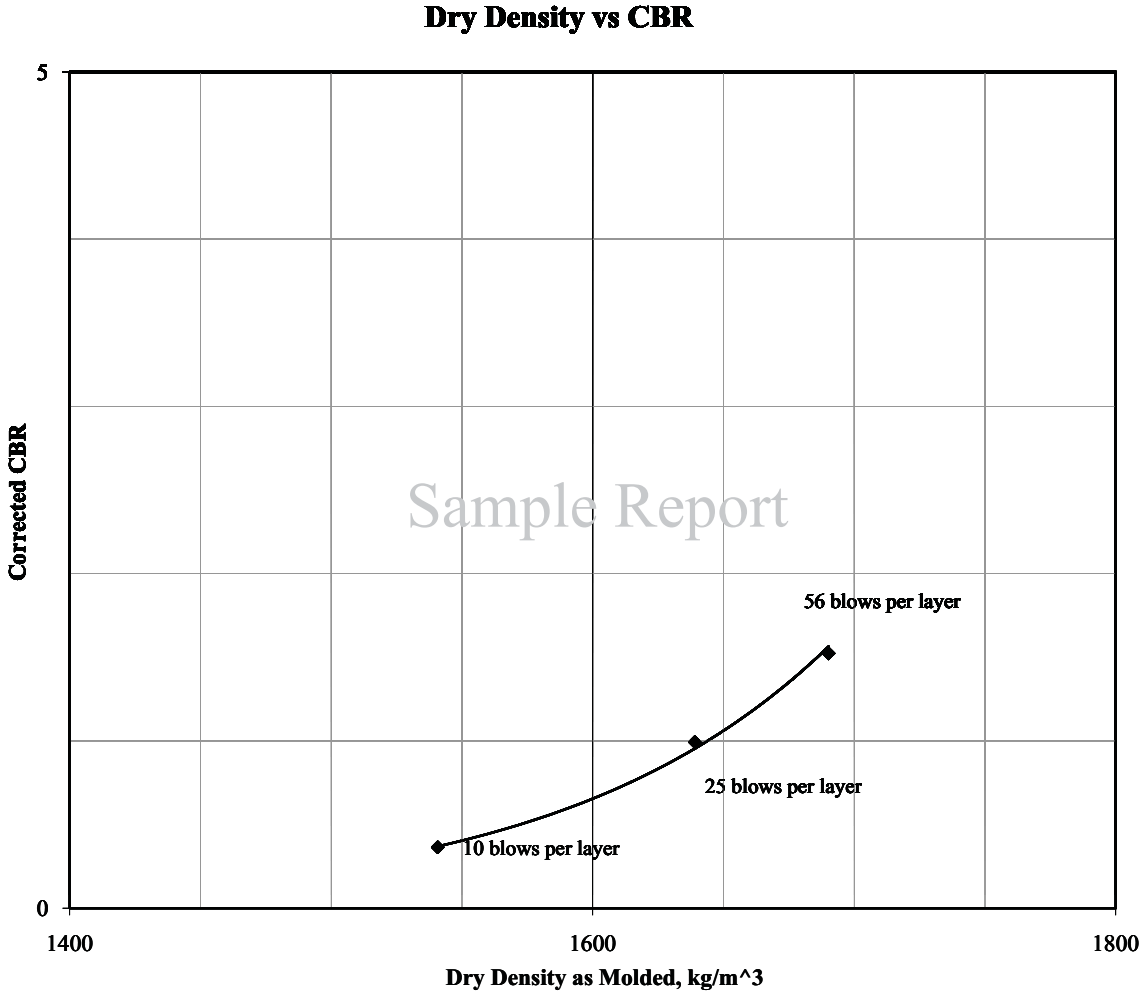
CALIFORNIA BEARING RATIO

ASTM D 1883-05 (One Water Content only)

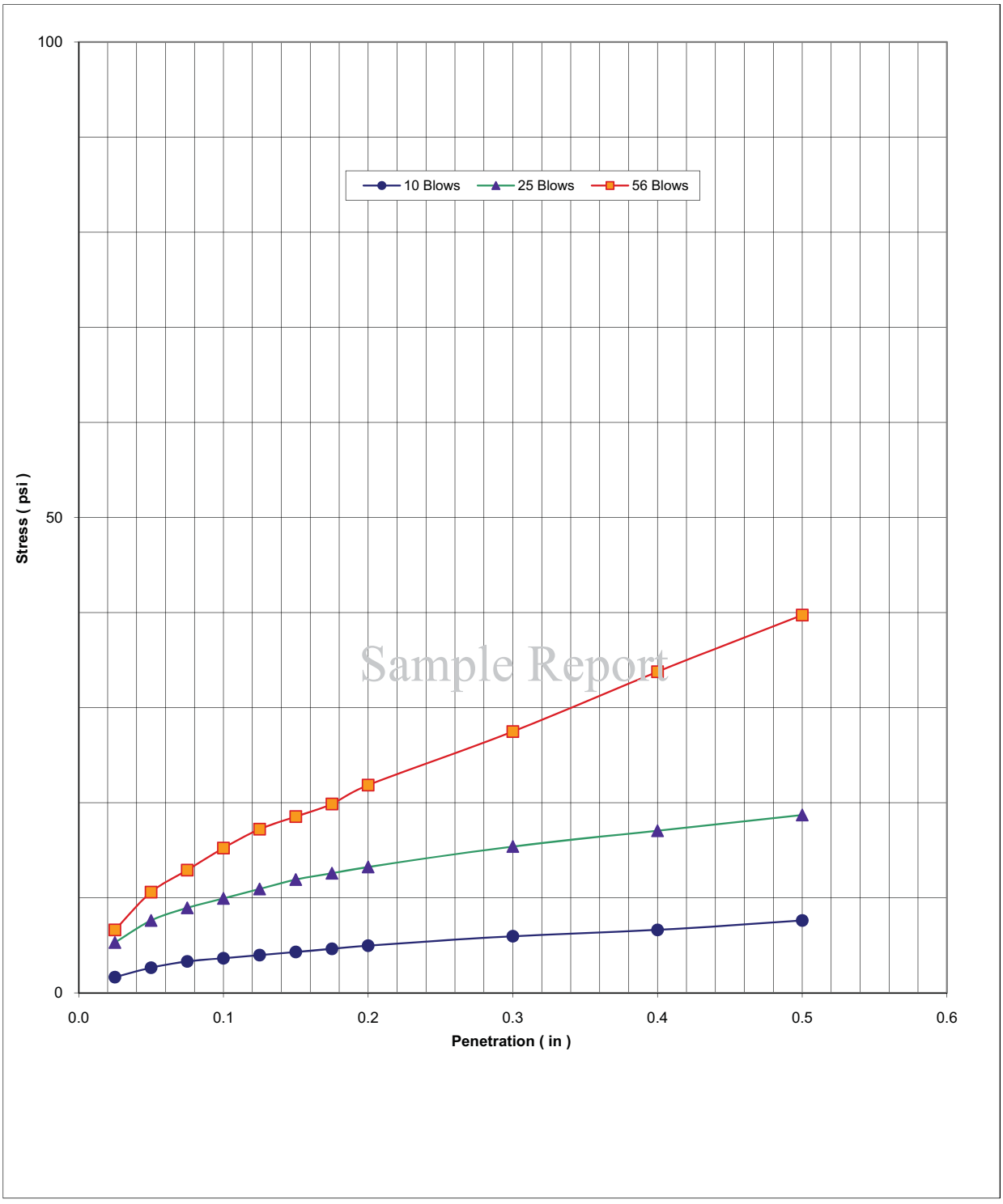
TP-1
Brown Lean Clay (CL)
Surcharge Amount: 4.54 Kg.

95% Optimum Moisture Content

	10 Blows	25 Blows	56 Blows
Dry density before soak, kg/m^3	1,541	1,639	1,690
Dry density after soak, kg/m^3	1,554	1,649	1,698
Moisture content before soak, %	19.5	19.8	19.8
Moisture content after soak, avg., %	27.4	25.3	23.7
Moisture content after soak, top 1", %	29.5	27.1	25.1
% Swell after 96 hour soak	-0.83	-0.61	-0.48
Bearing Ratio, 0.100" penetration	0.4	1.0	1.5



Stress vs. Penetration @ 95% Optimum Moisture Content

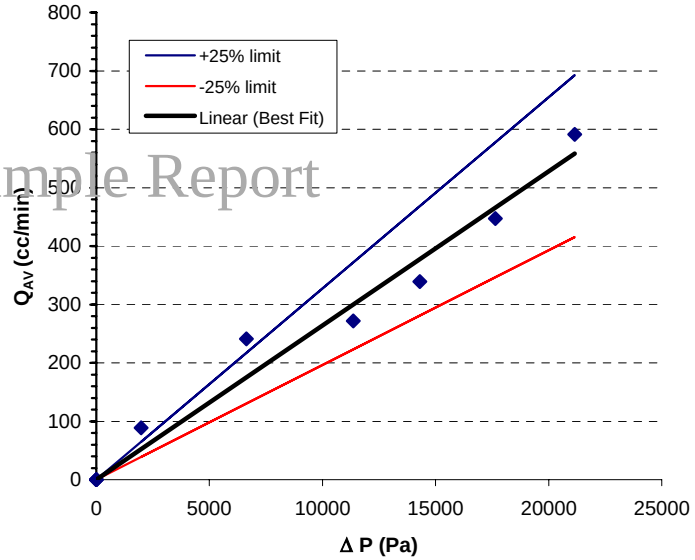


SOLUM CONSULTANTS LTD.
GEOTECHNICAL & MATERIAL TESTING LABORATORY

Measurement of Pneumatic Permeability of Partially Saturated Porous Material (ASTM D6539)

Client:	*XYZ Environmental Ltd.*	**Borehole No.:** *N/A*	**Tested by:** *KC / SF*
Project No.:	*12345678*	**Sample No.:** *BH09-01*	
Project Name:	*X Site Assessment*	**Depth:** *4.5-5.0 m*	
Location:	*X Site*	**Date :** *15-Oct-09*	*(dd -mm -yy)*

Experiment No.	TEMP. (deg. C)	Inlet Pressure (Pa)	Outlet Pressure (Pa)	Pressure Drop (Pa)	Bubble Meter Reading (cc/min)	Qav (cc/min)	Pneumatic Conductivity Kp (Darcy)	O-Outlier V-Valid
1	21.0	2250	270	1980	90	8.89E+01	1.89E-01	O
2	21.0	7200	570	6630	250	2.41E+02	1.53E-01	O
3	21.0	12560	1200	11360	290	2.72E+02	1.01E-01	V
4	21.0	16450	2150	14300	370	3.39E+02	9.98E-02	V
5	21.0	21000	3360	17640	500	4.47E+02	1.07E-01	V
6	21.0	26000	4850	21150	680	5.91E+02	1.18E-01	V
						Average:	1.1E-01	



Test Method:	√ Flexible Wall Rigid Wall	**Test Gas:**	√ N_2 Air
Barometric Pressure (kPa):	102.4	**Specimen Type:**	Undisturbed
Wet Mass:	420.36 g	**Sample Height :**	4.78 cm
Water Content:	18.6%	**Sample Diameter :**	7.17 cm
Wet Density:	2178.0 kg/m^3	**Sample Area :**	40.38 cm^2
Dry Density:	1836.5 kg/m^3	**Sample Volume :**	193.00 cm^3

Remarks:

Standard Test Method for Determine the Thermal Conductivity/Resistivity of Soils (ASTM D5334)

SOLUM™ CONSULTANTS LTD.

GEOTECHNICAL & MATERIAL TESTING LABORATORY

Project Number:	N/A		
Client:	N/A		
Project Name:	N/A		
Location:	---		
Sample ID:	TP-1	Depth:	Bulk
Tested By:	KC/SF	Reviewed By:	SF
Date of Testing:	9-Mar-10	(dd-mmm-yy)	

Results

Specimen No.	1	2	3	4	5	6				
Moisture Content (%)	0.7	1.7	3.4	5.0	9.2	13.5				
Dry Density (kg/m^3)	1725.0	1737.0	1750.0	1760.0	1758.0	1765.0				
Thermal Resistyvity (k-cm/watt)	427	266	164	129	86	78				
Temperature (^{o}C)	24.3	21.8	21.8	20.1	20.4	19.1				

Proctor Information:

Max. Dry Density (kg/m^3):	1855.0
Opt. Moisture Content (%):	14.2
Target % Compaction:	95%
Target Dry Density(kg/m^3):	1762.3